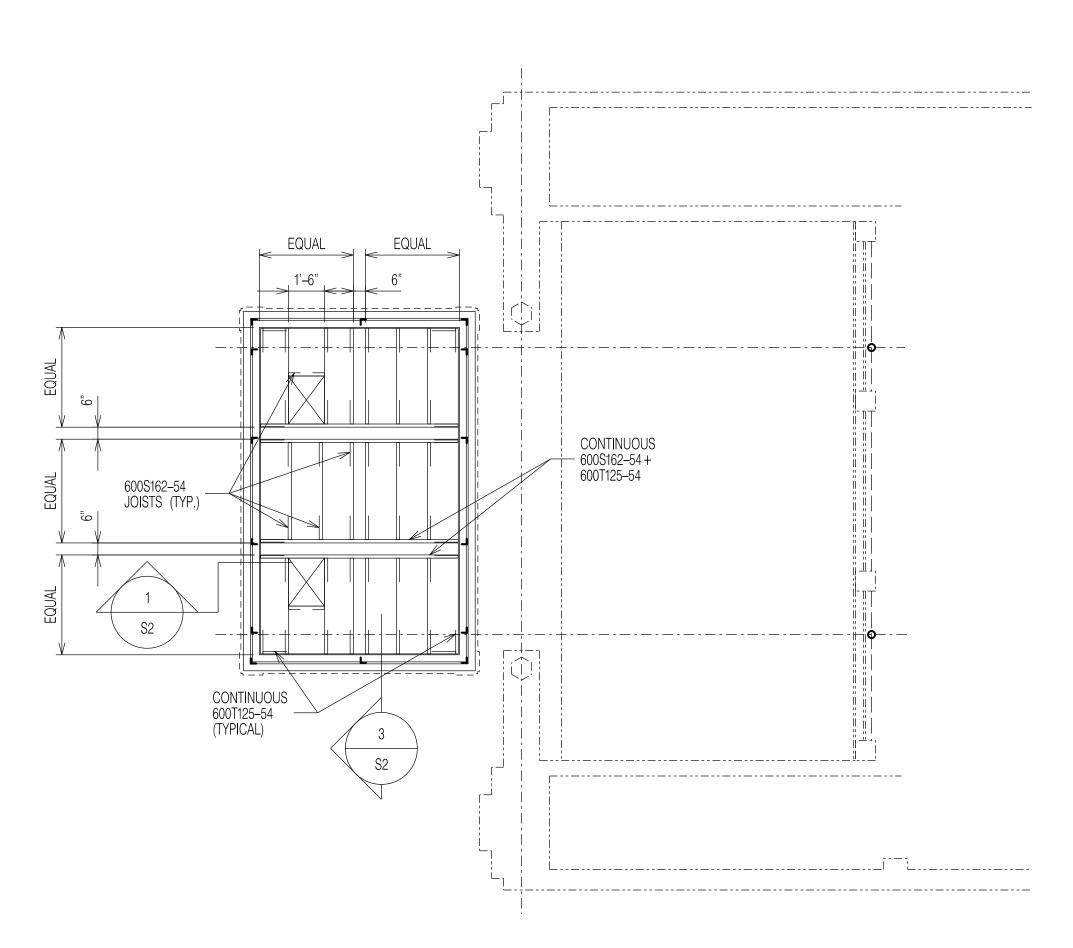
1 CANOPY FRAMING PLAN S1 SCALE: 1/4"=1'-0"



2 CANOPY CEILING FRAMING PLAN
SCALE: 1/4"=1'-0"

CANOPY FRAMING PLAN NOTES:

- 1. REMOVE THE EXISTING CANOPY FRAMING TO THE FACE OF THE EXISTING CONCRETE.
- 2. THE CANOPY ROOF DECK SHALL BE 1 1/2" DEEP 20 GAGE WIDE RIB GALVANIZED METAL DECK.

GENERAL NOTES

CODE

A. ALL CONSTRUCTION SHALL CONFORM TO THE PROVISIONS OF THE INTERNATIONAL BUILDING CODE/2015.

DESIGN LOADS

A. THE MINIMUM DESIGN SUPERIMPOSED LOADS FOR ALL NEW FRAMING IS AS FOLLOWS:

LIVE LOADS CANOPY ROOF	30 PSF
ROOF SNOW LOAD Pg Pf Ce I Ct	25 PSF 21 PSF 1.0 1.0 1.2
WIND BASIC WIND SPEED (3-SEC GUST) RISK CATEGORY EXPOSURE Cpi	115 MPH II B 18 AND +.
EARTHQUAKE RISK CATEGORY SEISMIC IMPORTANCE FACTOR SS S1 SITE CLASS SDS	II 1.0 0.141 0.043 D 0.151

STRUCTURAL AND MISCELLANEOUS STEEL

SEISMIC DESIGN CATEGORY

A. ALL STRUCTURAL AND MISCELLANEOUS STEEL SHALL CONFORM TO THE FIFTEENTH EDITION OF THE AISC "SPECIFICATION FOR THE DESIGN, FABRICATION AND ERECTION OF STRUCTURAL STEEL FOR BUILDINGS" AND ALL ITS SUPPLEMENTS, AND TO THE AISC "CODE OF STANDARD PRACTICE FOR STEEL BUILDINGS AND BRIDGES".

0.069

- B. ALL STRUCTURAL STEEL SHALL CONFORM TO ASTM A36 UNLESS NOTED OTHERWISE. ALL WIDE FLANGE SECTIONS SHALL CONFORM TO ASTM A992.
- C. STEEL PIPES SHALL CONFORM TO ASTM A53 TYPE E OR S GRADE B.
- D. ANCHOR BOLTS SHALL CONFORM TO ASTM F1554.
- E. ALL WELDED CONNECTIONS SHALL BE DONE WITH E70XX ELECTRODES. ALL WELDING SHALL CONFORM TO THE AMERICAN WELDING SOCIETY STANDARD CODE FOR "WELDING IN BUILDING CONSTRUCTION".
- F. ALL BOLTED CONNECTIONS SHALL USE 3/4" DIAMETER MINIMUM A-325N HIGH STRENGTH BOLTS. ALL CONNECTIONS SHALL DEVELOP THE FULL UNIFORM LOAD CAPACITY THE BEAM CAN CARRY WITH DUE CONSIDERATION TO CONCENTRATED LOADS AT THE ENDS OF THE MEMBER. NO CONNECTION SHALL BE MADE WITH LESS THAN TWO BOLTS.
- G. GROUT SHALL BE HIGHSTRENGTH NON-SHRINK AND NON-METALLIC.
- H. ALL STRUCTURAL STEEL SHALL BE SHOP PAINTED WITH AN APPROVED CORROSION RESISTANT PRIMER.
- I. COMPLETE SHOP DRAWINGS DETAILING ALL STRUCTURAL STEEL SHALL BE SUBMITTED TO THE ARCHITECT FOR REVIEW.
- J. THE CONTRACTOR SHALL EMPLOY AN INDEPENDENT INSPECTION AGENCY APPROVED BY THE ARCHITECT TO INSPECT THE STEEL AT BOTH THE SHOP AND THE FIELD.
- K. FIELD VERIFY (F.V.) EXISTING CONDITIONS AND DIMENSIONS PRIOR TO FABRICATING STEEL.

LIGHTGAGE STRUCTURAL METAL FRAMING

- A. LIGHTGAGE STRUCTURAL METAL FRAMING SHALL CONFORM TO THE LATEST EDITION OF THE SPECIFICATION FOR THE "DESIGN OF COLD-FORMED STEEL STRUCTURAL MEMBERS" AS PUBLISHED BY THE AMERICAN IRON AND STEEL INSTITUTE.
- B. COLD-FORMED METAL FRAMING SHALL BE FABRICATED FROM GALVANIZED-STEEL SHEET CONFORMING TO ASTM A653 G90 WITH A 33,000 PSI MINIMUM YIELD STRENGTH.
- C. LIGHTGAGE STUDS SHALL HAVE A MINIMUM THICKNESS OF 18 GAGE AND A MINIMUM FLANGE OF 1 5/8".
- D. LIGHTGAGE MEMBER DESIGNATIONS ARE IN ACCORDANCE WITH THE SECTIONS DEFINED BY THE STEEL STUD MANUFACTURERS ASSOCIATION (SSMA).
- E. THE DESIGN OF LIGHTGAGE FRAMING AND CONNECTIONS SHALL BE SIGNED AND SEALED BY A PROFESSIONAL ENGINEER REGISTERED IN THE STATE OF MARYLAND AND SUBMITTED TO THE ARCHITECT FOR REVIEW.
- F. SHOP DRAWINGS COMPLETE WITH MATERIAL DATA, ERECTION PLANS AND ALL CONNECTION DETAILS SHALL BE SUBMITTED FOR REVIEW.
- G. THE INSPECTOR FOR THE STRUCTURAL STEEL SHALL INSPECT THE LIGHT GAGE METAL FRAMING IN THE FIELD.

METAL DECK

- A. THE METAL DECK SHALL BE FABRICATED AND ERECTED IN ACCORDANCE WITH THE "SPECIFICATIONS AND COMMENTARY FOR STEEL ROOF DECK" AS PUBLISHED BY THE STEEL DECK INSTITUTE
- B. THE METAL DECK SHALL BE WELDED TO EACH SUPPORT WITH PUDDLE WELDS SPACED AT 12" MAXIMUM ON CENTER.
- C. WELDS SHALL BE CLEANED AND PAINTED WITH A ZINC RICH PRIMER.
- D. THE INSPECTOR FOR THE STRUCTURAL STEEL SHALL INSPECT THE METAL DECK INSTALLATION.

POST-INSTALLED ANCHORS

- A. HOLES FOR POST-INSTALLED ANCHORS SHALL BE CLEANED PRIOR TO THE INSTALLATION OF THE ANCHOR IN ACCORDANCE WITH THE MANUFACTURER'S RECOMMENDATIONS.
- B. THE EMBEDMENT SHOWN FOR POST-INSTALLED ANCHORS IS THE NOMINAL EMBEDMENT DEFINED BY HILTI AFTER INSTALLATION.

GLEASON ASSOCIATES, INC ARCHITECTS

> Arch Social Club Marquee 2426 Pennsylvania Avenue

*Date:*March 6, 2020

Revisions:

Sheet Title:

PLANS
AND
GENERAL
NOTES

Project Number: Drawn By: Checked By:

004-19 NC KR

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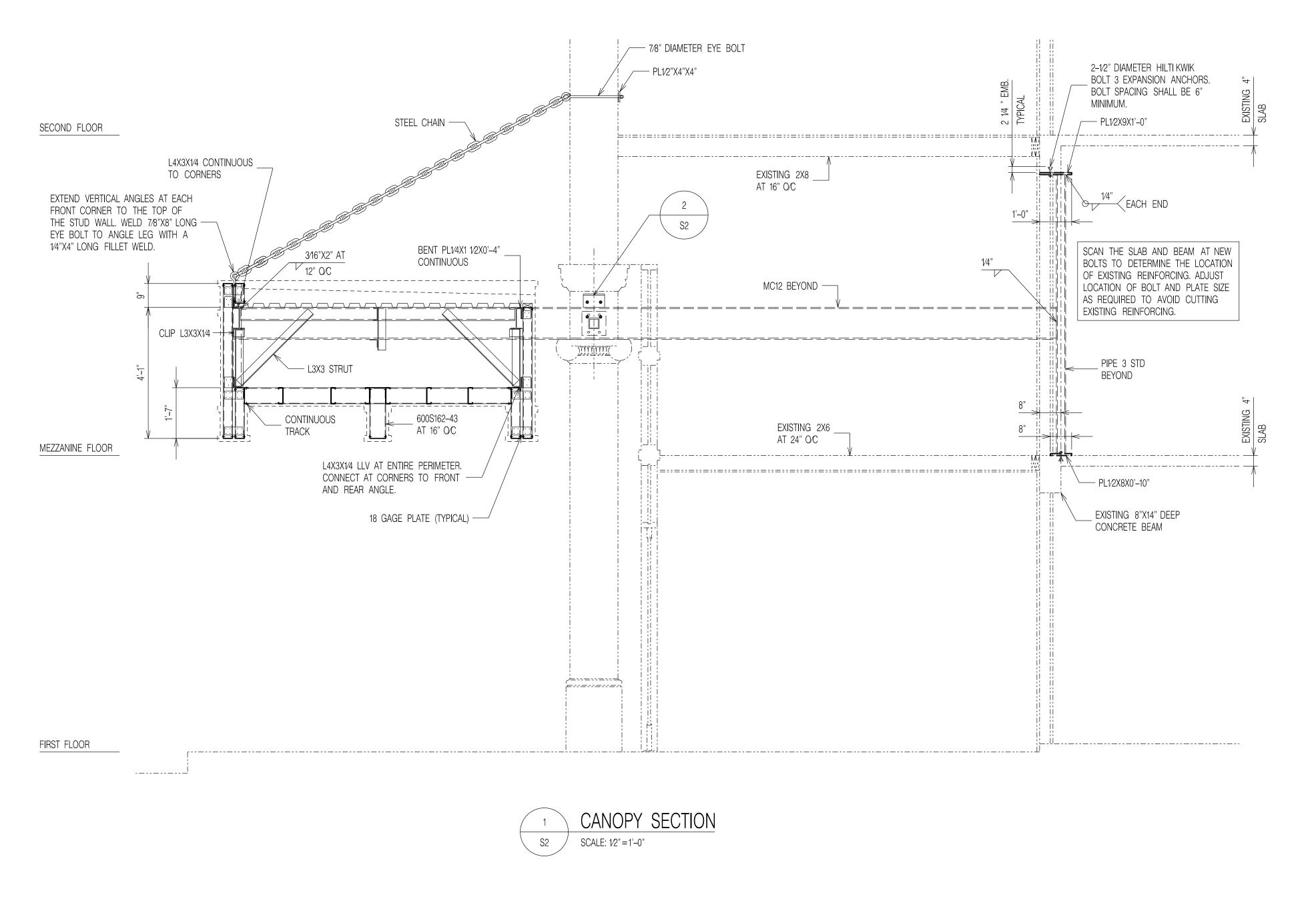
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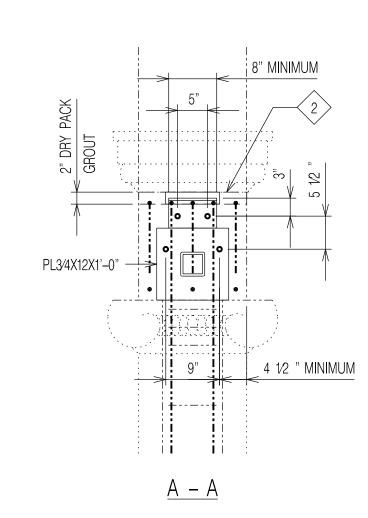
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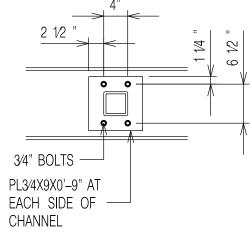


7" FIELD VERIFY 3/4" DIAMETER HAS-R STAINLESS STEEL BOLT IN HIT-RE 500 V3 ---ADHESIVE BY HILTI (TYPICAL) TOP OF EXISTING 16" WIDE GALVANIZED CONCRETE BEAM (F.V.) L5X3 1/2X3/4 ______ EXISTING COLUMN _____ **ENCLOSURE** 9" EMBEDMENT TYPICAL EXISTING 10" DIAMETER CONCRETE COLUMN FIELD VERIFY (F.V.)



- 1. SCAN THE EXISTING CONCRETE BEAM TO DETERMINE THE LOCATIONS OF EXISTING REINFORCING.
- 2. CUT A SLOT IN THE EXISTING CONCRETE BEAM AS SHOWN. THE SLOT AND NEW ANGLE SHALL BE THE FULL WIDTH OF THE BEAM OR AS LONG AS POSSIBLE WITHOUT CUTTING EXISTING REINFORCING. NOTIFY THE ARCHITECT IF THE EXISTING REINFORCING PROHIBITS THE INSTALLATION OF THE MINIMUM LENGTH ANGLE SHOWN.
- 3. BOLT THE NEW ANGLE AND CHANNEL TO THE EXISTING BEAM AS SHOWN. SUBMIT ALTERNATE BOLT LOCATIONS IF REQUIRED TO AVOID CUTTING EXISTING REINFORCING TO THE ARCHITECT FOR





Design Develop Pennsylvani Social Club Arch 2426]

ARCHITECT

March 6, 2020

Revisions:

SECTIONS AND DETAILS

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